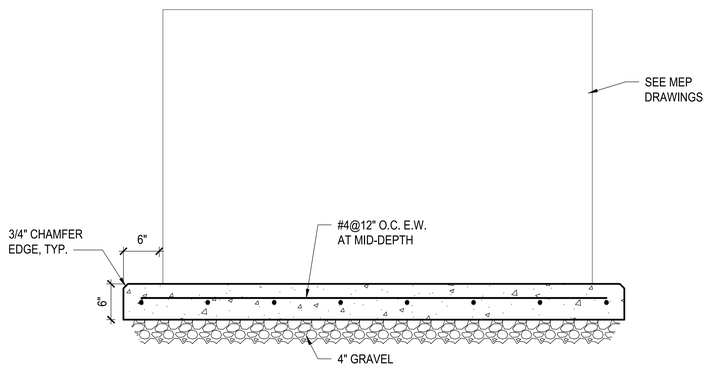
FOOTING SCHEDULE						
ALLOWABLE SOIL BEARING PRESSURE = 2,000 PSF						
MARK	LENGTH	WIDTH	THICKNESS	BAR QTY.	BAR #	REMARKS
F2	2'-0"	2'-0"	12"	2	5	BOTT. REINF. EA. WAY
F2.5	2'-6"	2'-6"	12"	3	5	BOTT. REINF. EA. WAY
F3	3'-0"	2'-0"	12"	3	5	BOTT. REINF. EA. WAY



1 S001 TYP. EQUIPMENT PAD DETAIL
SCALE: 1"=1'-0"

DESIGN NOTES

I. DESIGN LOADS FOR NEW WORK

A. ROOF SNOW LIVE LOAD
1. Pg = 30 PSF
2. Pf = 21 PSF + DRIFTING, MIN ROOF DESIGN LOAD = 30 PSF
3. ROOF LIVE LOAD = 20 PSF + 300 LBS. CONCENTRATED LOAD.
4. SNOW EXPOSURE FACTOR, Ce = 1.0
5. SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
6. SLOPE FACTOR, Cs = 0.9
7. THERMAL FACTOR, Ct = 1.2

B. WIND LOAD
1. Vult (3-second gust) = 115 MPH
2. Vasd = 89 MPH
3. EXPOSURE = B
4. INTERNAL PRESSURE COEFFICIENT = 0.18GCpi
5. COMPONENT AND CLADDING PRESSURE PER ASCE 7-10, TABLE 30.3-1 AND FIGURES 30.4-1 to 4.
6. FLAT ROOF UPLIFT PRESSURE = 26 PSF

C. SEISMIC LOAD
1. RISK CATEGORY = II
2. SEISMIC IMPORTANCE FACTOR, IE = 1.0
3. MAPPED SPECTRAL ACCELERATION, SHORT PERIOD, Ss = 0.124
4. MAPPED SPECTRAL ACCELERATION, 1-SEC. PERIOD, S_1 = 0.051
5. SITE CLASS = D
6. NO DESIGN REQUIRED PER IRC/R301.2.2

D. CODE: THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE/2015 AND THE 2015 VIRGINIA USBC.

E. ASSUMED SOIL PARAMETERS
1. AT REST EARTH PRESSURE= 60H
2. ACTIVE EARTH PRESSURE = 45H
3. PASSIVE EARTH PRESSURE COEFFICIENT, Kp = 3.00
4. PASSIVE EARTH PRESSURE= 3.0 X 125 = 375 PCF
5. MODULUS OF SUBGRADE REACTION = 100 PCI
6. FRICTION COEFFICIENT = 0.30
7. SOIL UNIT WEIGHT = 125 PCF

F. DEAD LOADS
1. SUPERIMPOSED ROOF = 10 PSF (SELF WT. NOT INCLUDED)

II. ROOF SHEATHING

A. ROOF SHEATHING SHALL BE 5/8-INCH, CDX, APA RATED SHEATHING, EXPOSURE I, PER THE "AMERICAN PLYWOOD ASSOCIATION." SHEATHING SHALL BE FASTENED WITH 8d NAILS AT 6-INCHES ON CENTER AT PANEL EDGES AND AT 12-INCHES ON CENTER AT ALL INTERMEDIATE SUPPORTS.

B. NAILS INDICATED IN THE NOTES SHALL BE DEFINED AS 8d=0.131"x2.5". SUBSTITUTIONS FOR THESE NAIL SIZES SHALL BE SUBMITTED IN WRITING FOR APPROVAL.

C. ALL ROOF SHEATHING SHALL BE LAID CONTINUOUSLY BETWEEN THE EDGES OF THE ROOF.

III. CONCRETE

A. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH ACI 301, ACI 318 AND ACI 302.

B. CEMENT SHALL COMPLY WITH ASTM C150, TYPE I OR II.

C. REINFORCING STEEL SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60. ALL REINFORCEMENT SPLICES SHALL BE A MINIMUM OF 40 BAR DIAMETERS.

D. CAST IN PLACE CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH F'c AS FOLLOWS:
1. FOOTINGS = 3000 PSI
2. PIERS = 4500 PSI (A/E)

E. CONCRETE SLUMP SHALL = 4" ± 1".

F. MINIMUM CONCRETE COVER BETWEEN FACE OF REINFORCING BAR AND FACE OF CONCRETE SHALL BE AS FOLLOWS:
1. CONCRETE CAST AGAINST EARTH = 3"
2. FORMED CONCRETE EXPOSED TO WEATHER OR EARTH = 2"

G. ALL PIERS AND EXTERIOR EXPOSED SLABS SHALL HAVE A MINIMUM AIR ENTRAINMENT OF 6% ± 1.5% PER ACI- 318 4.2.1.

H. SHOP DRAWINGS FOR ALL CONCRETE REINFORCEMENT SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL.

IV. STRUCTURAL STEEL

A. ALL STRUCTURAL STEEL SHALL BE ASTM FABRICATED AND ERECTED IN ACCORDANCE WITH AISC "STEEL CONSTRUCTION MANUAL" WITH A MINIMUM YIELD STRENGTH AS FOLLOWS:
1. W SHAPES: Fy = 50 ksi, PER ASTM A 992.
2. ANGLES AND RODS: Fy = 36 ksi PER ASTM A36.
3. PLATES: Fy = 50 ksi, PER ASTM A572 GRADE 50.
4. HSS SHAPES (SQUARE/RECTANGULAR): Fy = 50 ksi PER ASTM A-500 GRADE C.
5. ANCHOR RODS: Fy = 55 ksi PER ASTM F1554 GRADE 55 - SUPPLEMENT S1.
6. BOLTS: Fy = 120 ksi PER ASTM F3125 GRADE A325.
7. NUTS: ASTM A563

B. ALL EXTERIOR EXPOSED BOLTS SHALL BE HOT-DIPPED GALVANIZED CONFORMING TO ASTM A153, CLASS C.

C. WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE "STRUCTURAL WELDING CODE" AWS D1.1-2015. USE 70 KSI, LOW-HYDROGEN ELECTRODES.

D. NO FABRICATION SHALL PROCEED PRIOR TO SHOP DRAWINGS APPROVAL.

E. NO OPENINGS IN BEAMS OR COLUMNS ARE PERMITTED WITHOUT ARCHITECT'S APPROVAL.

F. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ARCHITECT AS TO LOCATION, TYPE OF SPLICE AND CONNECTION TO BE MADE.

G. ALL EXTERIOR EXPOSED STRUCTURAL STEEL AS DESIGNATED IN PLANS SHALL BE HOT-DIPPED GALVANIZED (1.5OZ./SF.) TO ASTM A123 GRADE 65. TOUCH UP ALL DAMAGED AREAS PER ASTM A780. TOUCH-UP MATERIALS ARE REQUIRED TO MEET A COATING THICKNESS OF AT LEAST 2.0 MILS (50.8 µm) FOR ONE APPLICATION, AND THE FINAL COATING THICKNESS OF THE REPAIR AREA IS DICTATED BY THE MATERIAL USED TO DO THE REPAIR. BLEMISHES IN THE FIELD TO DECKING PANELS WILL BE CAUSE FOR REPLACEMENT.

H. ALL OTHER STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH A MODIFIED ALKYD RUST INHIBITIVE PRIMER, 2.5 TO 3.5 MILS DFT (BASIS OF DESIGN IS TNEMEC SERIES 10).

I. STRUCTURAL STEEL SHOP DRAWINGS SHALL INCLUDE DETAILS FOR APPLICATION AND ASSEMBLY OF ALL STRUCTURAL MEMBERS. INCLUDE DETAILS OF CUTS, CONNECTIONS, HOLES, AND OTHER PERTINENT DATA. INDICATE WELDS BY STANDARD AWS 2.1 SYMBOLS SHOWING SIZE, LENGTH AND TYPE OF EACH WELD. SHOP DRAWINGS SHALL BE SUBMITTED TO ARCHITECT FOR APPROVAL PRIOR TO FABRICATION.

J. ALL STEEL BEAM CONNECTIONS SHALL BE SNUG-TIGHT, SIMPLE SHEAR BEARING TYPE CONNECTIONS.

K. ALL WORK SHALL COMPLY WITH THE AISC CODE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

V. POST-INSTALLED ANCHORS

A. EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES AS PROVIDED BY HILTI INC. OR AN EQUIVALENT AS APPROVED BY THE STRUCTURAL ENGINEER.
1. ANCHORAGE TO MASONRY:
a. ADHESIVE ANCHORS FOR USE IN GROUT FILLED CMU, HOLLOW CMU, BRICK W/HOLES AND MULTI-WYTHE BRICK.
- HILTI HIT-HY 270 ADHESIVE SYSTEM (OR EQUAL) PER ICC ESR-4143
- INSTALLED USING THE SAFE SET DRILLING METHOD
- THREADED RODS: HILTI HIT-Z
b. ADHESIVE ANCHORS SHALL CURE A MINIMUM OF 20-HOURS PRIOR TO ANY LOADS BEING APPLIED TO THE ANCHORS.

B. SUBSTITUTION REQUESTS FOR ALTERNATE POST-INSTALLED ANCHOR PRODUCTS MUST BE APPROVED IN WRITING PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR CORRESPONDING ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION MUST ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.

C. INSTALL ANCHORS PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING. EXPANSION/ADHESIVE ANCHORS SHALL BE INSTALLED SUCH THAT THE APPLIED SHEAR FORCES ACT THROUGH THE BOLT SHAFT, NOT THE THREADS. TAKE MEASURES TO AVOID DRILLING OR CUTTING OF EXISTING REINFORCING STEEL. BLOW HOLES CLEAN PRIOR TO SETTING ANCHORS.

D. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

E. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN THE ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCE INDICATED ON THE DRAWINGS.

VI. STRUCTURAL COLD-FORMED STEEL

A. ALL WORK SHALL CONFORM TO THE STANDARDS OF THE "AMERICAN IRON AND STEEL INSTITUTE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBER" - LATEST EDITION.

B. ALL WELDING SHALL BE IN ACCORDANCE WITH THE "AMERICAN WELDING SOCIETY D.1.3, STRUCTURAL WELDING CODE FOR SHEET STEEL." MIN. 14 GAUGE MEMBERS SHALL BE USED AT WELDED CONNECTIONS. ALL WELDS SHALL BE TOUCHED UP WITH ZINC RICH PAINT.

C. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL SIGNED AND SEALED BY THE CONTRACTOR'S REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF VIRGINIA TO THE ARCHITECT DEPICTING:
1. CROSS-SECTIONS, PLANS AND ELEVATIONS.
2. CONNECTION DETAILS SHOWING REQUIRED SCREWS/WELDS.
3. FLOOR TO FLOOR ELEVATIONS.
4. DIMENSIONS.
5. BRIDGING LOCATIONS.

D. ALL MATERIALS SHALL BE GALVANIZED AND COLD FORMED OF STEEL CONFORMING TO ASTM A570 GRADE D FOR JOISTS. ASTM A1003 FOR TRACKS AND STUDS. MINIMUM 18 GAUGE (NO EXCEPTIONS.)

E. ALL NON-LOAD BEARING STUDS LOCATED BENEATH STEEL BEAMS SHALL BE PROVIDED WITH A VERTICAL DEFLECTION TRACK DESIGNED FOR A BEAM DEFLECTION OF L/360.

F. WALL STUD BRACING SHALL BE INSTALLED AT THIRD POINTS IN ALL BEARING PARTITIONS; AT MID-HEIGHT IN NON-LOAD BEARING PARTITIONS.

G. ALL LIGHT GAUGE FRAMING SHALL BE DESIGNED BY THE MANUFACTURER'S ENGINEER FOR THE CODE REQUIRED LOADS. SHOP DRAWINGS SHALL BE PREPARED UNDER AND STAMPED BY THE CONTRACTOR'S PROFESSIONAL ENGINEER REGISTERED LICENSED IN THE STATE OF VIRGINIA SUBMITTED TO ARCHITECT FOR APPROVAL.

VII. STEEL DECKING

A. CONTRACTOR SHALL PROVIDE MATERIALS, DESIGN AND INSTALLATION OF DECK FOR THE REQUIREMENTS OF STEEL DECK INSTITUTE'S "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS AND ROOF DECKS." DECK PROPERTIES SHALL BE COMPUTED USING THE LATEST EDITION OF AISI SPECIFICATION FOR THE "DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."

B. STEEL ROOF DECKING SHALL BE WIDE RIB, GALVANIZED STEEL, CONFORMING TO ASTM A653 STRUCTURAL QUALITY WITH MINIMUM YIELD STRENGTH = 33 KSI. GALVANIZING SHALL CONFORM TO ASTM A924 COATING CLASS G90.

C. STEEL FORM DECK SHALL BE PAINTED CONFORMING TO ASTM A1008 WITH A MINIMUM YIELD STRENGTH = 60 KSI. SIXTEEN GAUGE WELDING WASHERS SHALL BE USED AT ALL WELDED CONNECTIONS.

D. U.N.O ON DRAWINGS, ALL DECKING SHALL BE WELDED TO STRUCTURAL STEEL MEMBERS WITH 5/8" PUDDLE @ 12-INCHES O.C. (36/4 PATTERN)

E. U.N.O ON DRAWINGS, SIDELAPS FASTENERS FOR METAL DECK DIAPHRAGM SHALL BE #10 TEK SCREWS SPACED AT 3-FEET ON CENTER OR ½ THE SPAN LENGTH OF THE DECK, WHICHEVER IS MORE FREQUENT.

F. STEEL DECK SHALL BE INSTALLED CONTINUOUS 3 SPANS MINIMUM (U.N.O.) AND SHALL BEAR AT LEAST 2" ON STEEL SUPPORTS.

G. DECKING CONTRACTOR SHALL FURNISH AND INSTALL CONTINUOUS CLOSURES AND POUR STOPS AT DECK ENDS, EDGES AND OPENINGS WHERE NO STEEL ANGLE IS SPECIFIED.

VIII. GENERAL

A. THE CONTRACTOR SHALL MEASURE AND PROVIDE ALL EXISTING FIELD DIMENSIONS, ELEVATIONS AND CONDITIONS AT THE JOB SITE PRIOR TO CONSTRUCTION AND THE SUBMISSION OF SHOP DRAWINGS AND SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.

B. DETAILS, SECTIONS, AND NOTES SHOWN ON THESE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR CONDITIONS ELSEWHERE UNLESS OTHERWISE SHOWN OR NOTED.

C. ALL WALLS ARE DESIGNED AS LATERALLY BRACED BY THE ROOF SYSTEMS. CONTRACTOR SHALL ENSURE THAT WALLS ARE ADEQUATELY BRACED DURING CONSTRUCTION.

D. ANY REQUIRED TEMPORARY SHORING SHALL BE IN CONFORMANCE WITH OSHA REGULATIONS. UNBRACED EXCAVATIONS SHALL BE SLOPED NO GREATER THAN (1.5) HORIZONTAL TO (1) VERTICAL.

E. CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN VICINITY OF FOUNDATIONS AND NOTIFY THE ARCHITECT IF A CONFLICT EXISTS. PROVIDE INFORMATION ON LOCATION SIZE AND ELEVATION OF UTILITIES PRIOR TO START OF WORK SO THAT ANY NECESSARY CHANGES CAN BE MADE WITHOUT DELAYING THE PROJECT SCHEDULE.

F. THE DEVELOPMENT AND IMPLEMENTATION OF JOB SITE SAFETY AND CONSTRUCTION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

G. CONTRACTOR SHALL PROVIDE INDEPENDENTLY PREPARED SHOP DRAWINGS AND SHALL NOT REPRODUCE ANY PORTION OF THE CONTRACT DOCUMENTS IN PREPARING SHOP DRAWINGS. THE SHOP DRAWINGS SHALL NOT SIMPLY BE A MARK-UP OF THE CONTRACT DOCUMENTS.

IX. TESTING AND INSPECTION

A. THE OWNER WILL RETAIN THE SERVICES OF AN INSPECTION AGENCY TO PERFORM THE FOLLOWING SERVICES, ADDITIONAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH LOCAL CODE REQUIREMENTS.

B. ALL WELDS ARE TO BE VISUALLY INSPECTED AND MEASURED.

C. THE PLACEMENT OF ALL CONCRETE REINFORCEMENT SHALL BE INSPECTED.

D. CONCRETE CYLINDERS SHALL BE TAKEN IN ACCORDANCE WITH LOCAL CODE REQUIREMENTS, IN ABSENCE OF LOCAL REQUIREMENTS, ONE SET OF 6 CYLINDERS SHALL BE TAKEN FOR EACH DAY'S POUR: (2) 7-DAY, (2) 28-DAY, (2) HOLD.

E. INSPECTION OF SUBGRADE BELOW ALL FOUNDATIONS AND SLAB ON GRADE TO VERIFY THE ADEQUACY OF THE BEARING MATERIAL.

F. WRITTEN REPORTS SHALL BE SUBMITTED TO THE ARCHITECT STATING COMPLIANCE OR NONCOMPLIANCE WITH DESIGN DOCUMENTS AND SPECIFICATIONS. ALL REPORTS SHALL BE SIGNED AND SEALED BY A REGISTERED ENGINEER LICENSED IN THE STATE OF VIRGINIA.

G. HIGH-STRENGTH BOLTS SHALL BE SNUG-TYPE AND SHALL BE VISUALLY INSPECTED PER THE REQUIREMENTS OF THE "SPECIFICATION FOR STRUCTURAL JOINTS (14th EDITION - AISC)."

X. EARTHWORK

A. ALLOWABLE SOIL BEARING PRESSURE FOR ALL SHALLOW FOOTINGS IS ASSUMED TO BE 2000 PSF. SHOULD UNSUITABLE MATERIAL BE ENCOUNTERED, FOOTINGS SHALL BE OVEREXCAVATED AND REPLACED WITH LEAN CONCRETE, F'c = 2000 PSI. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW EXTERIOR GRADE, UNLESS NOTED OTHERWISE. WORK SHALL BE COORDINATED WITH EXISTING UNDERGROUND UTILITIES IN ACCORDANCE WITH TYPICAL DETAIL./OVERCUT SHALL NOT UNDERMINE EXISTING ADJACENT FOUNDATIONS.

B. IF FOOTINGS ARE NOT TO BE POURED THE DAY OF EXCAVATION, FOOTING TRENCHES SHALL BE BACKFILLED WITH LEAN CONCRETE IMMEDIATELY UPON EXCAVATION TO PREVENT GROUNDWATER INFILTRATION.

C. FIELD MOISTURE CONTENTS SHALL BE MAINTAINED WITHIN 2% OF OPTIMUM DURING STRUCTURAL FILL COMPACTION. MOISTURE CONDITIONING SHOULD BE ANTICIPATED.

D. PRIOR TO PLACEMENT OF GRANULAR FILL LAYER, THE SUBGRADE BENEATH ALL SLAB ON GRADE SHALL BE PROOFROLLED, PROPERLY COMPACTED AND FREE OF STANDING WATER, MUD, AND FRONZEN SOIL.

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LE Project # 20-062 LE Project Engineer: JX

DULLES SOUTH RECREATION CENTER PHASE 5 - POOL FILTER MEDIA UPGRADES

24950 RIDING CENTER DR SOUTH RIDING, VA 20152

No.:	Revision:	Date:
	75% CD	08.10.20
	95% CD	09.04.20
	PERMIT	10.14.20
	FOR BID	02.01.21

Seal:

Sheet Title:

DESIGN NOTES

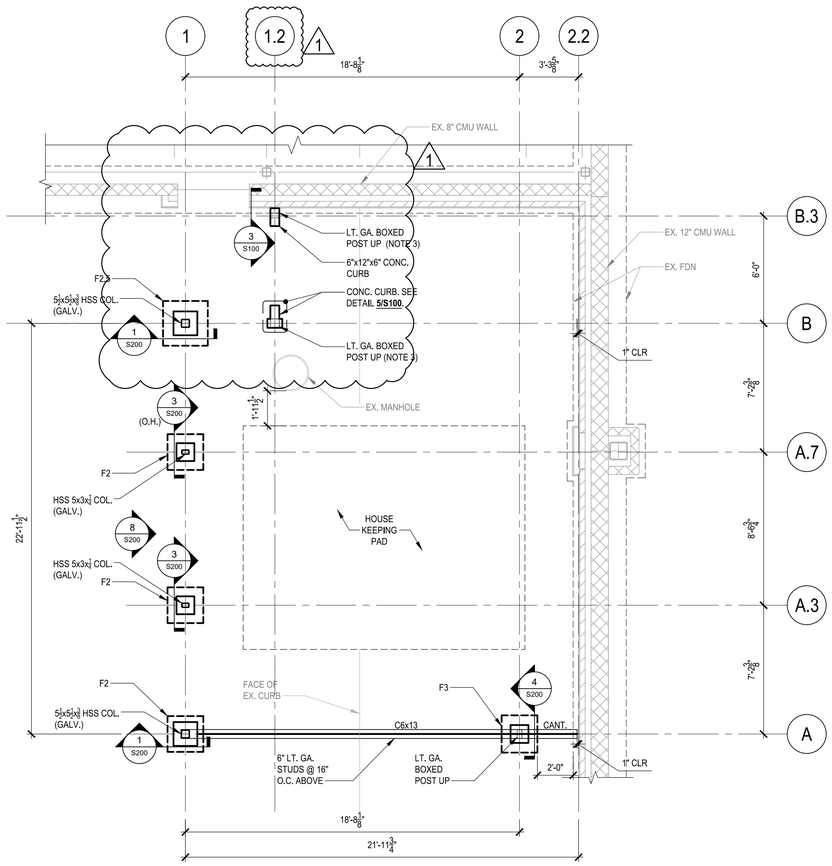
Graphic Scale: As Shown

Drawn By: JX | Checked by: DL

Sheet No.

S-001

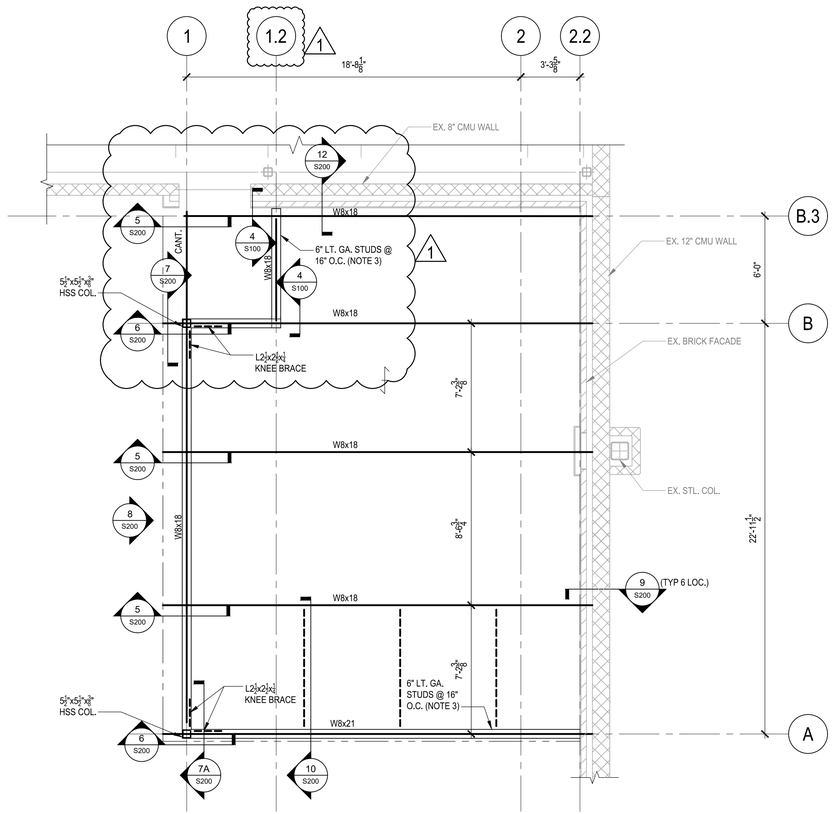
Print 24 X 36 for full scale



1 S100 FOUNDATION PLAN

SCALE: 1/4"=1'-0"

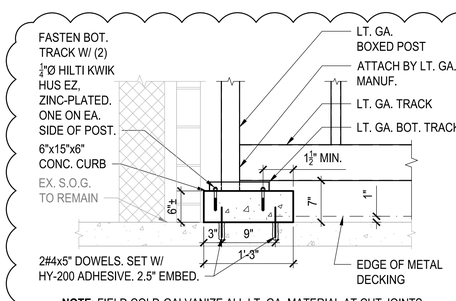
NOTES:
1. STOP WALL FTG. AT EX. CURB AND PROVIDE LT. GA. BOXED HEADER TO SUPPORT FRAMING BEYOND CURB.
2. SEE 1/S001 FOR HOUSE KEEPING PAD DETAIL.
3. FASTEN TRACK BENEATH LIGHT GAUGE BOXED POST TO CONCRETE BLOCK USING (2) ¼"Ø HILTI KWIK HUS EZ, ZINC-PLATED ANCHORS.



2 S100 ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"

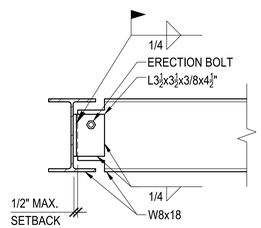
NOTES:
1. DESIGNATES 1½"x20 GA. METAL ROOF DECK
2. BP - DESIGNATES A 5"x5"x⅜" STL. BEARING PL PER DETAIL 8/S200.
3. BRACE STUDS LATERALLY AT THE TOP PER DETAIL 11/S200.



NOTE: FIELD COLD-GALVANIZE ALL LT. GA. MATERIAL AT CUT JOINTS

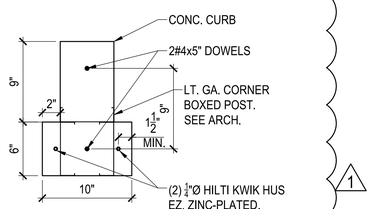
3 S100 LT. GA. SUPPORT ON EX. SLAB

SCALE: 1"=1'-0"



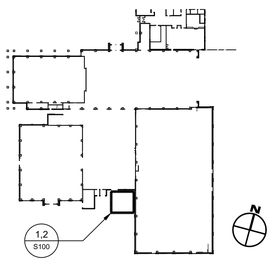
4 S100 BEAM TO BEAM CONN.

SCALE: 1 1/2"=1'-0"



5 S100 FASTENER LAYOUT @ CORNER

SCALE: 1 1/2"=1'-0"



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FOUNDATION AND FRAMING PLANS

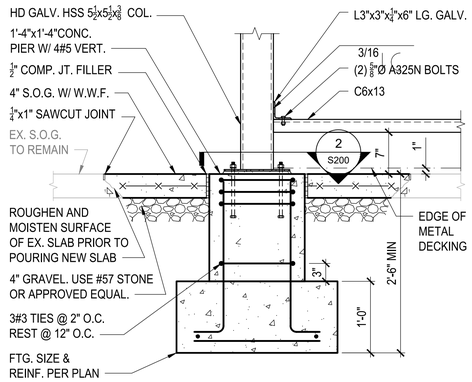
Graphic Scale: As Shown

Drawn By: JX | Checked by: DL

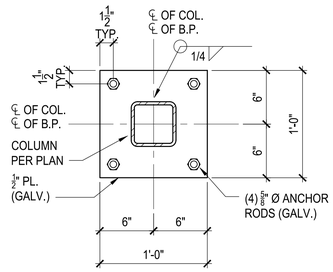
Sheet No.

S-100

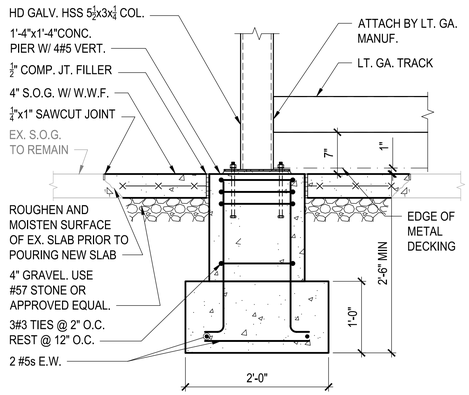
Print 24 X 36 for full scale



1 S200 (TYP) POST FOOTING
SCALE: 1"=1'-0"



2 S200 (TYP) BASE PLATE DETAIL
SCALE: 1 1/2"=1'-0"



3 S200 FTG. @ LT. GA. SUPPORT
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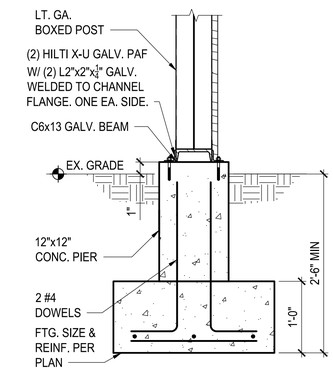

4 S200 SECTION @ LT. GA. POST
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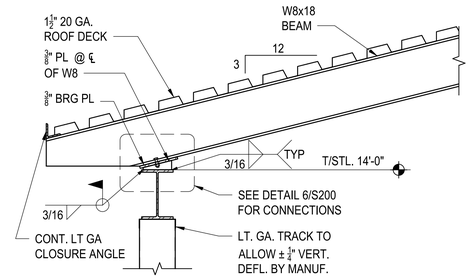

5 S200 (TYP) RAFTER CONNECTION
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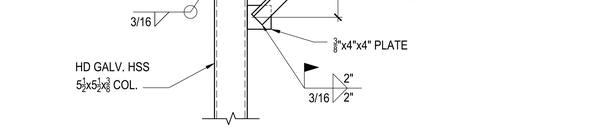
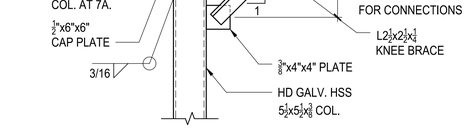
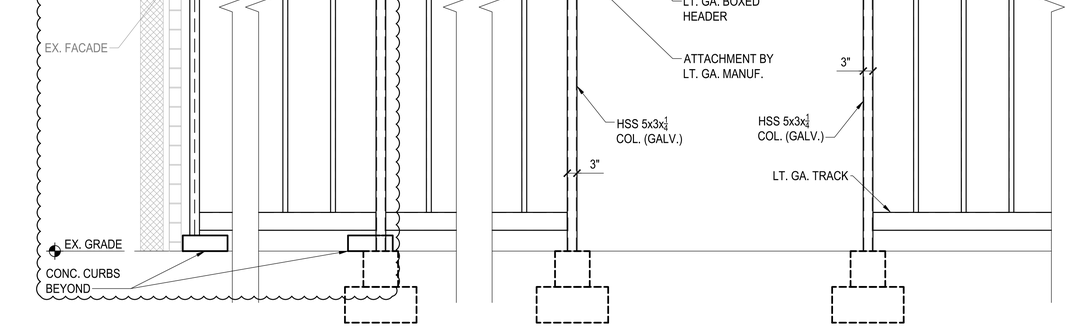


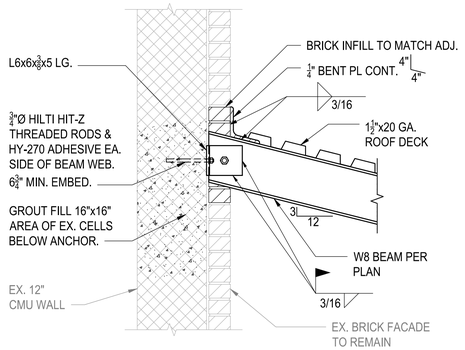
6 S200 BRACE CONNECTION DET.
SCALE: 1"=1'-0"



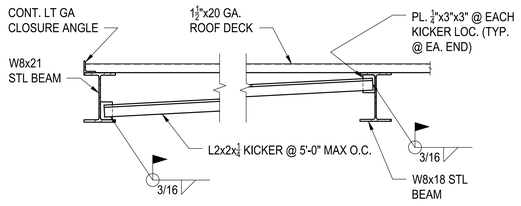
7, 7A S200 RAFTER CONNECTION PERP.
SCALE: 1"=1'-0"



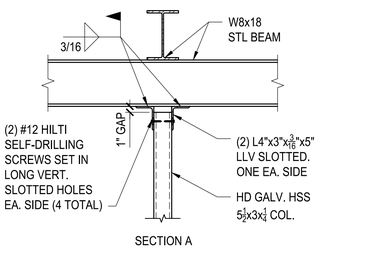
8 S200 ELEVATION @ WALL OPENING
SCALE: 1/2"=1'-0"



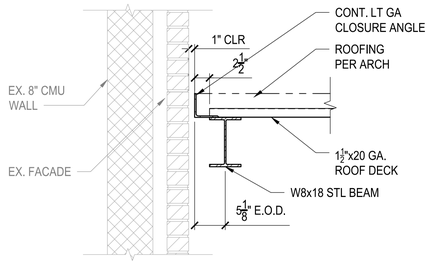
9 S200 SECTION @ BEAM BEARING ON EX. MASONRY WALL
SCALE: 1"=1'-0"



10 S200 RAFTER LATERAL BRACE DET.
SCALE: 1"=1'-0"



11 S200 COL. LATERAL CONX.
SCALE: 1"=1'-0"



12 S200 SECTION @ EDGE OF DECK
SCALE: 1"=1'-0"

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DULLES SOUTH RECREATION CENTER PHASE 5 - POOL FILTER MEDIA UPGRADES

24950 RIDING CENTER DR SOUTH RIDING, VA 20152

No.:	Revision:	Date:
	75% CD	08.10.20
	95% CD	09.04.20
	PERMIT	10.14.20
	FOR BID	02.01.21
1	ADDENDUM 1	XX.XX.21

Seal:

Sheet Title:

DETAILS AND SECTIONS

Graphic Scale: As Shown

Drawn By: JX | Checked by: DL

Sheet No.

S-200

Print 24 X 36 for full scale